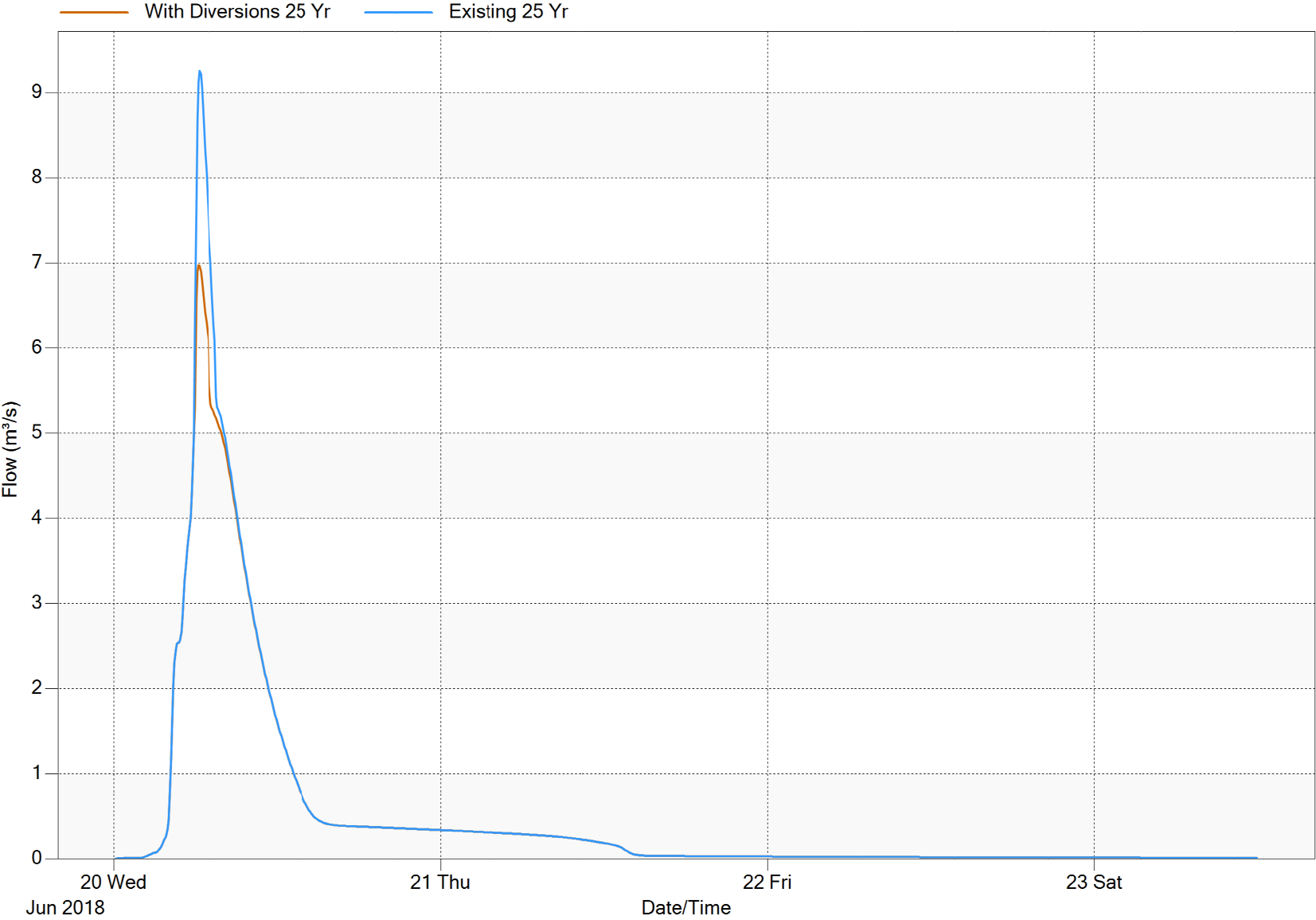
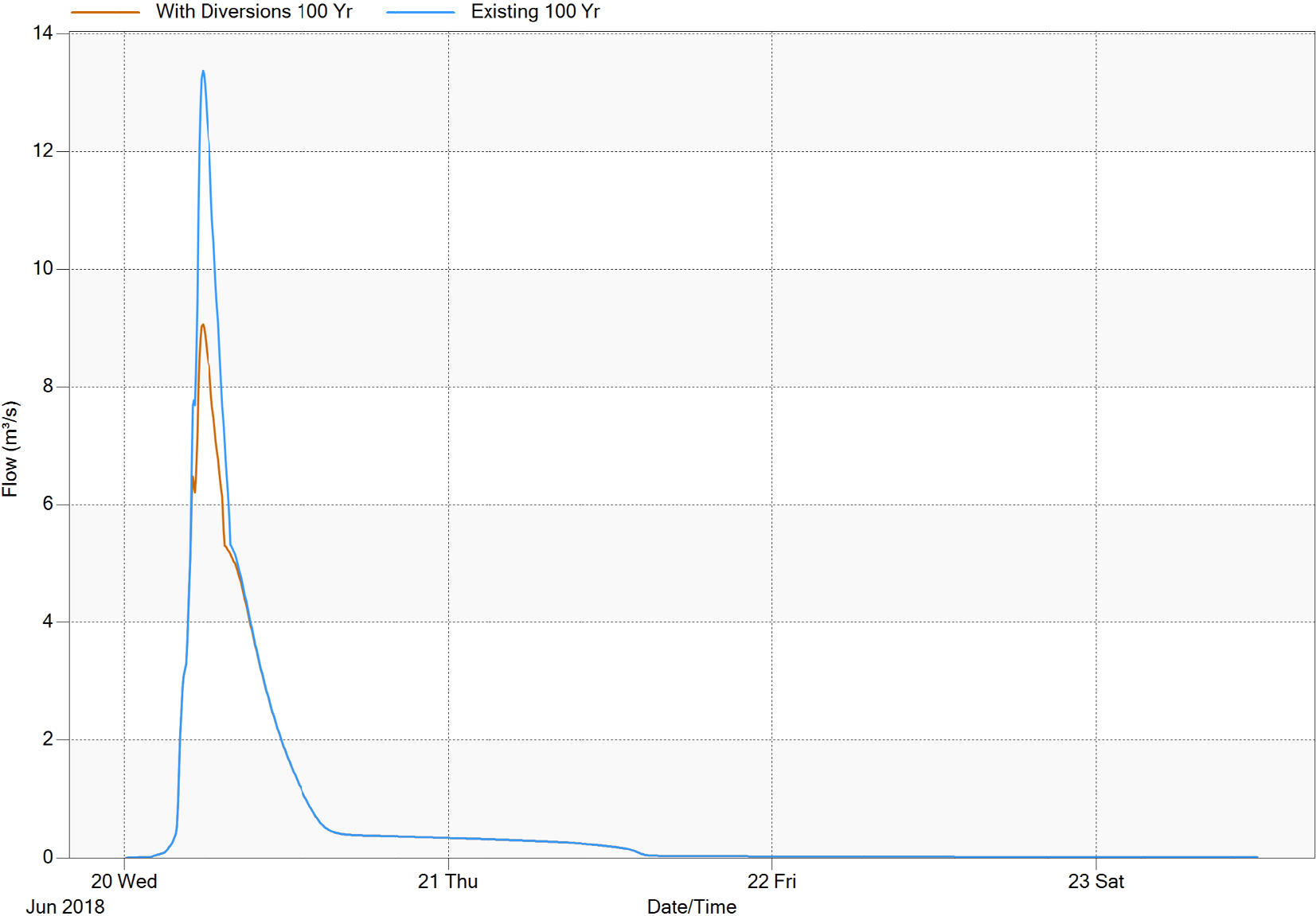


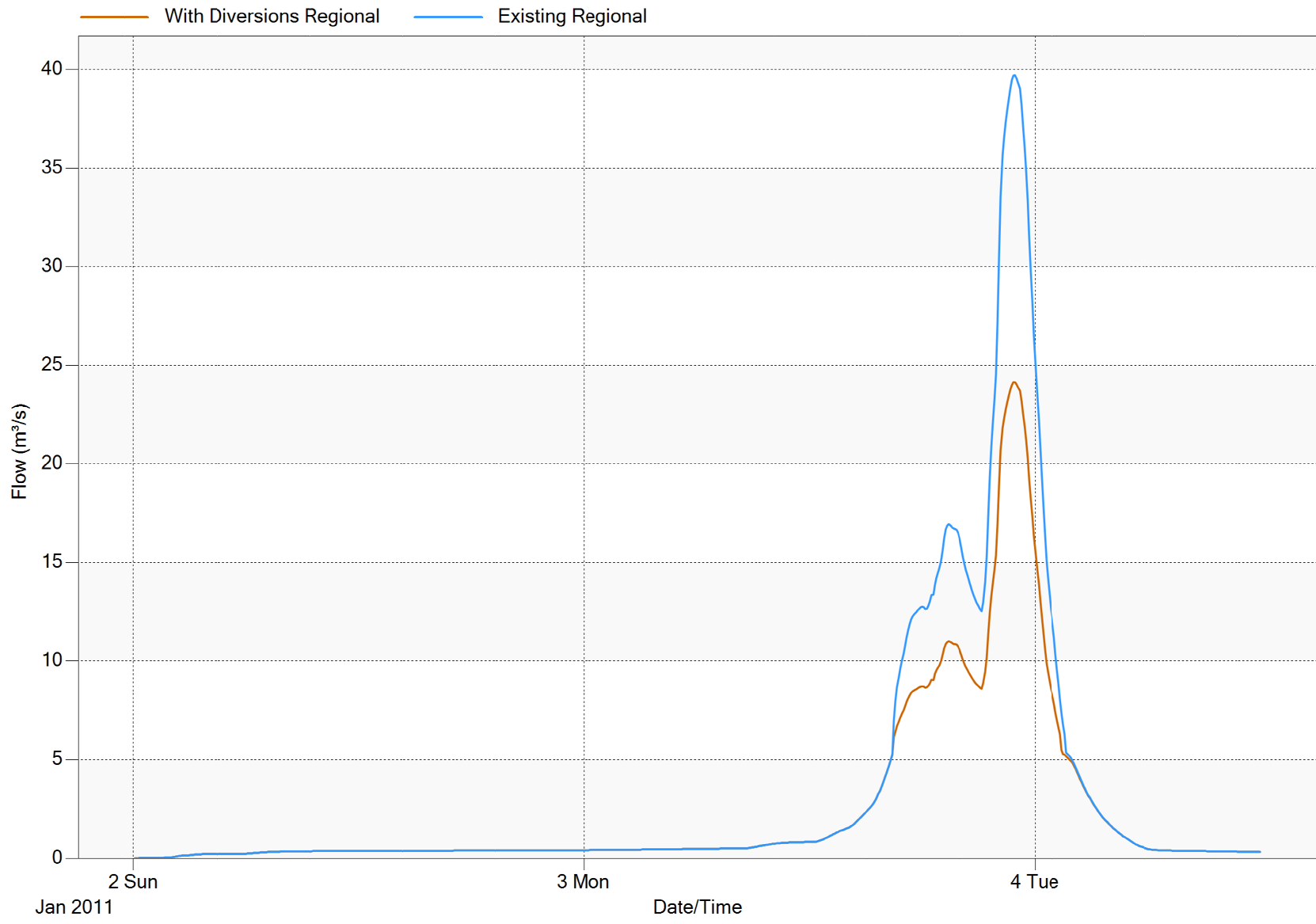
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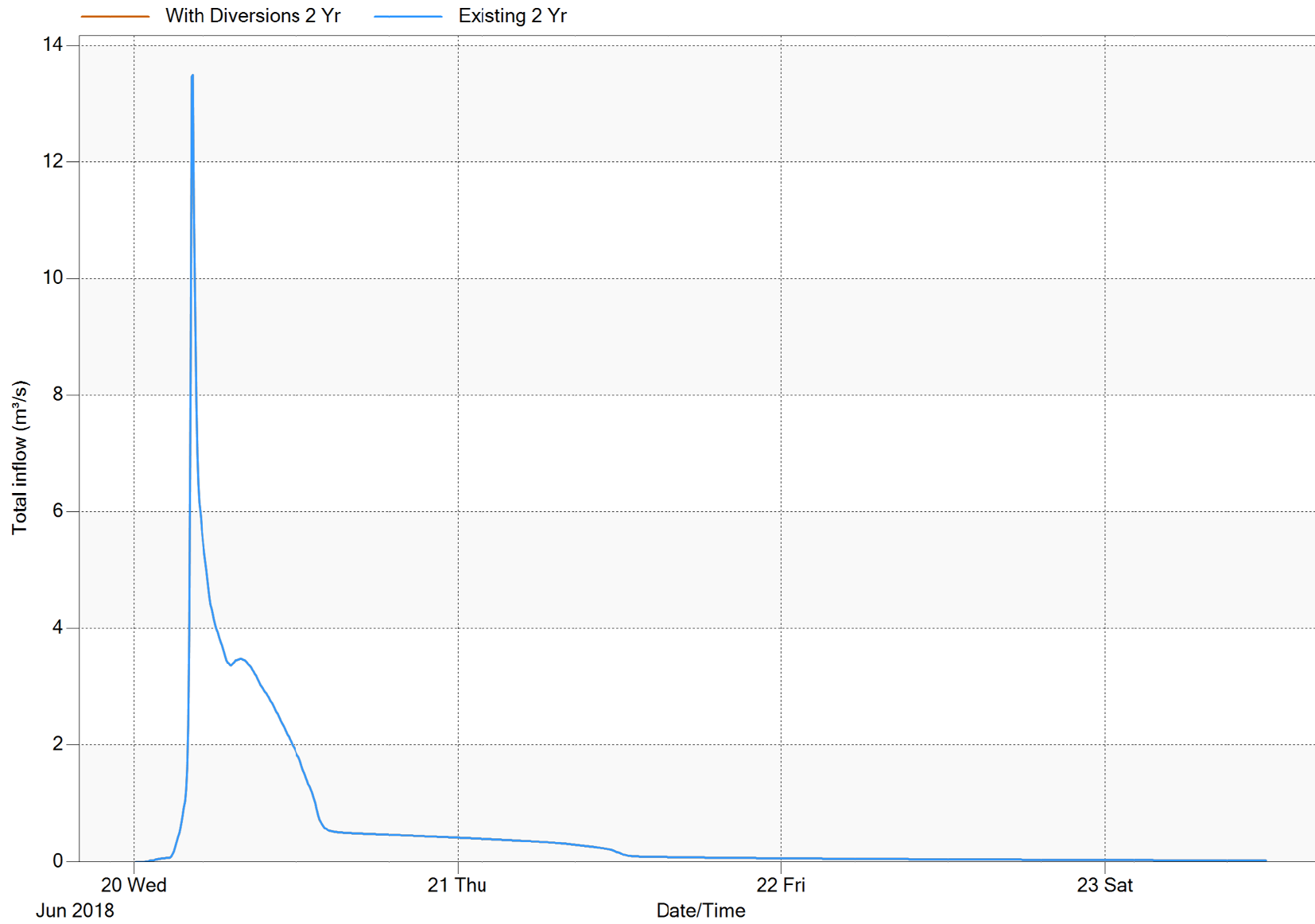
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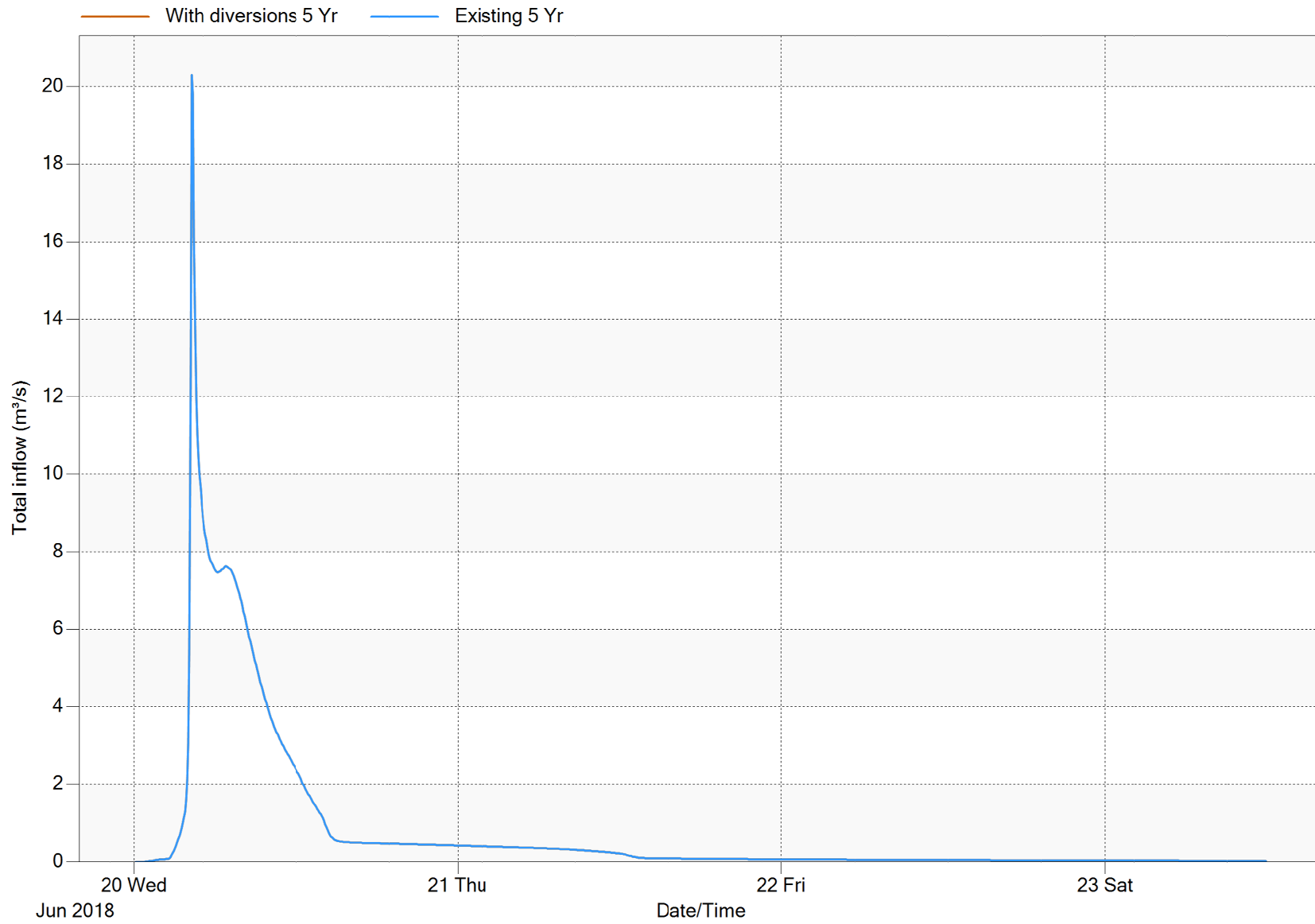
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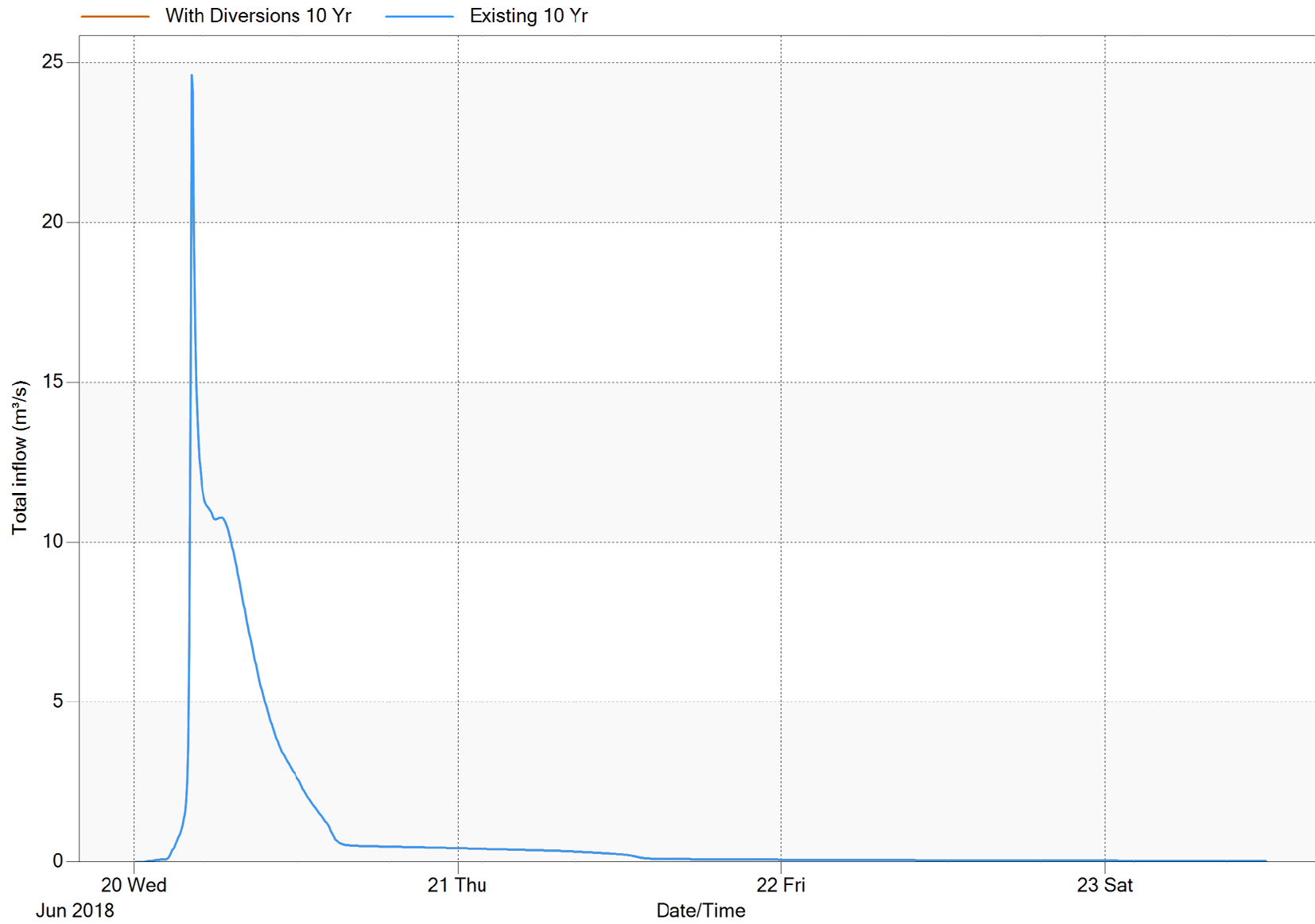
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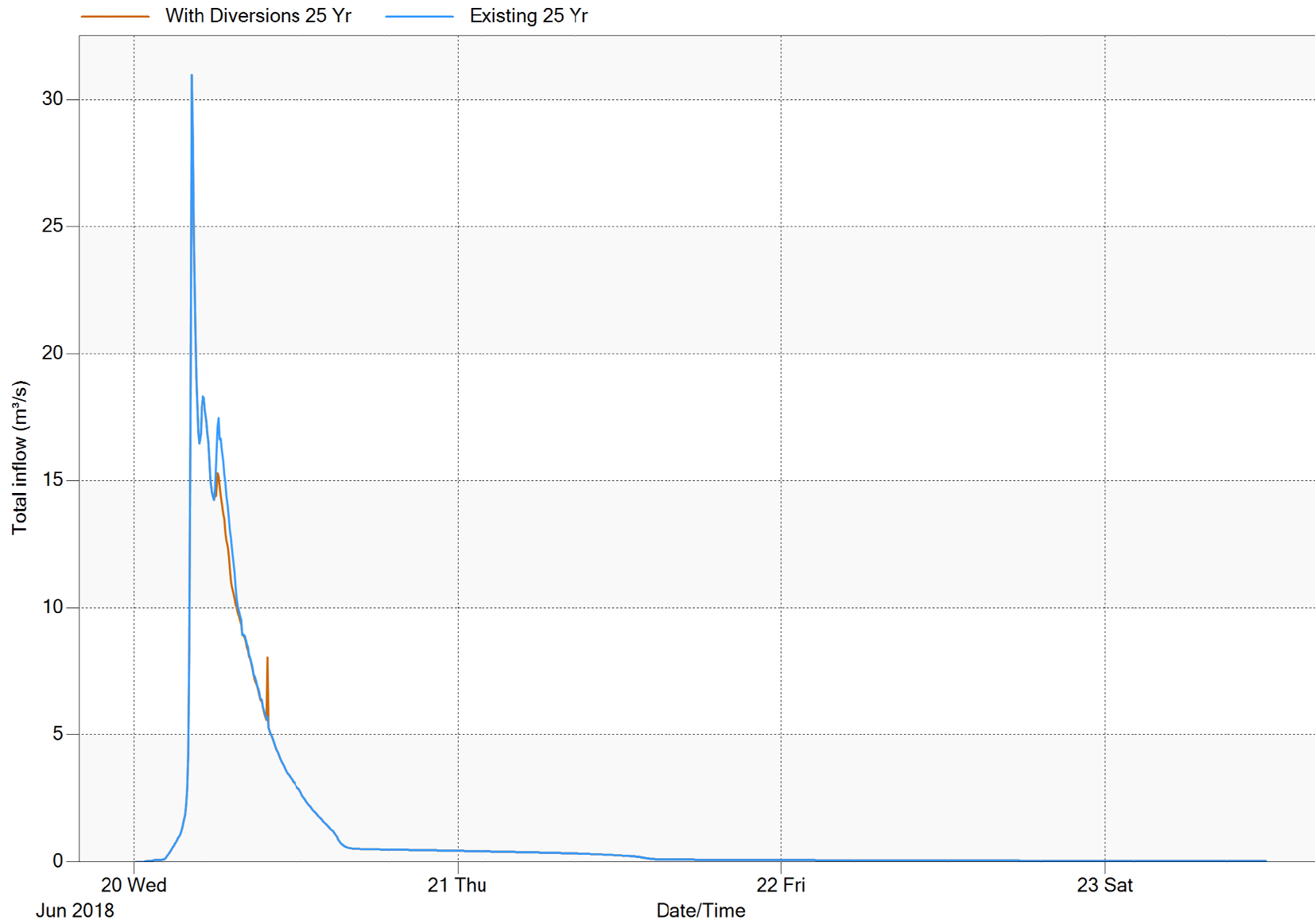
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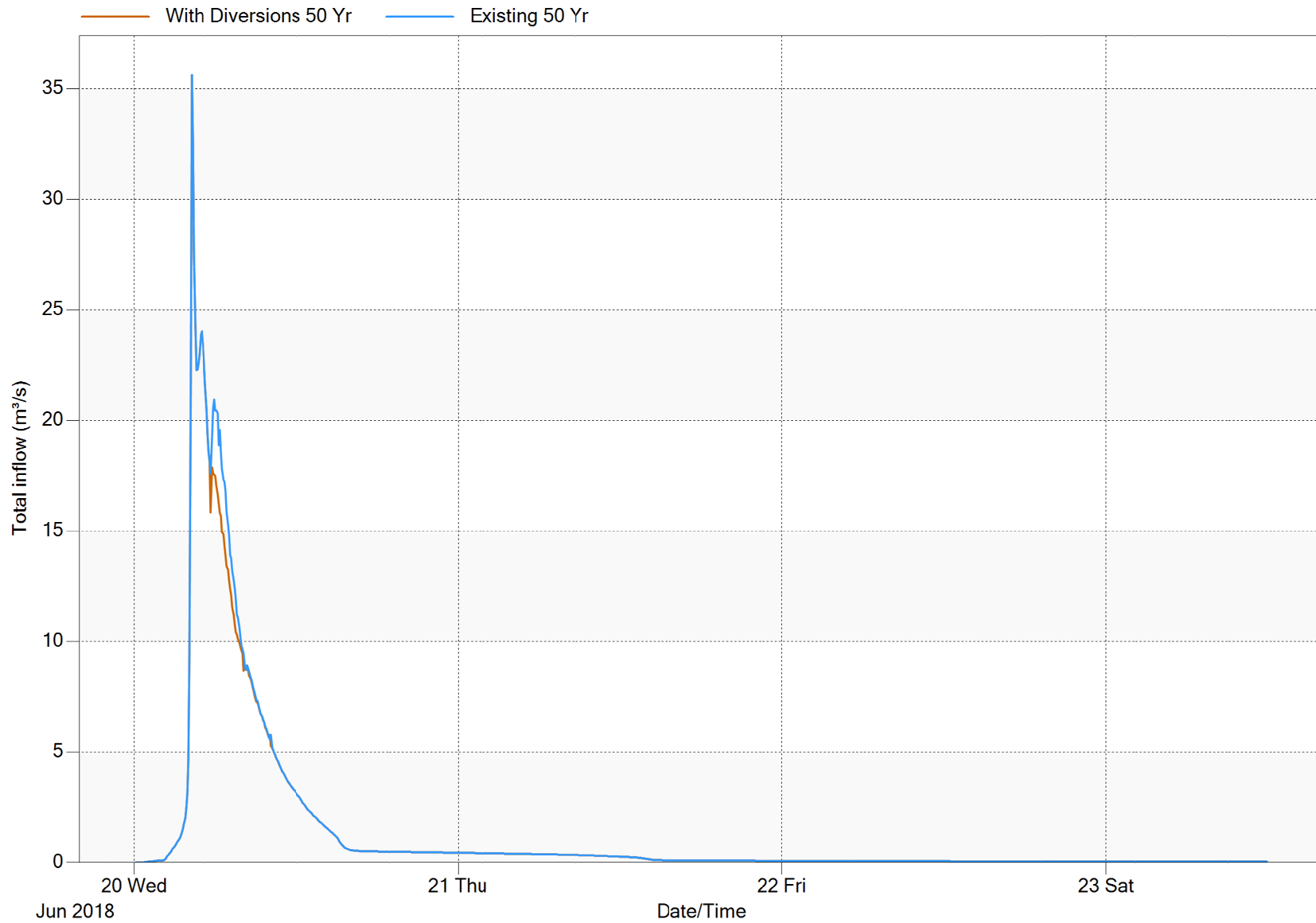
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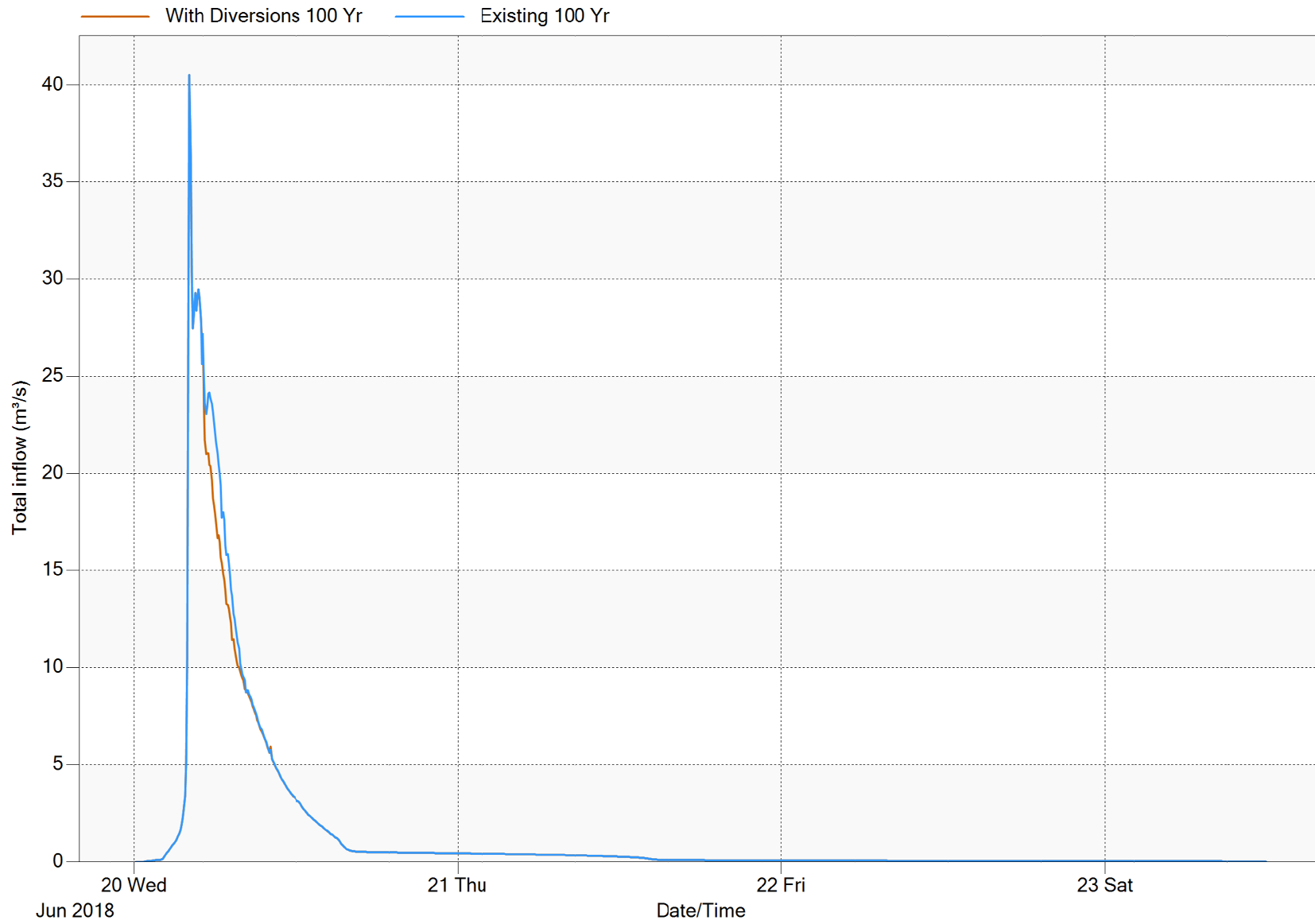
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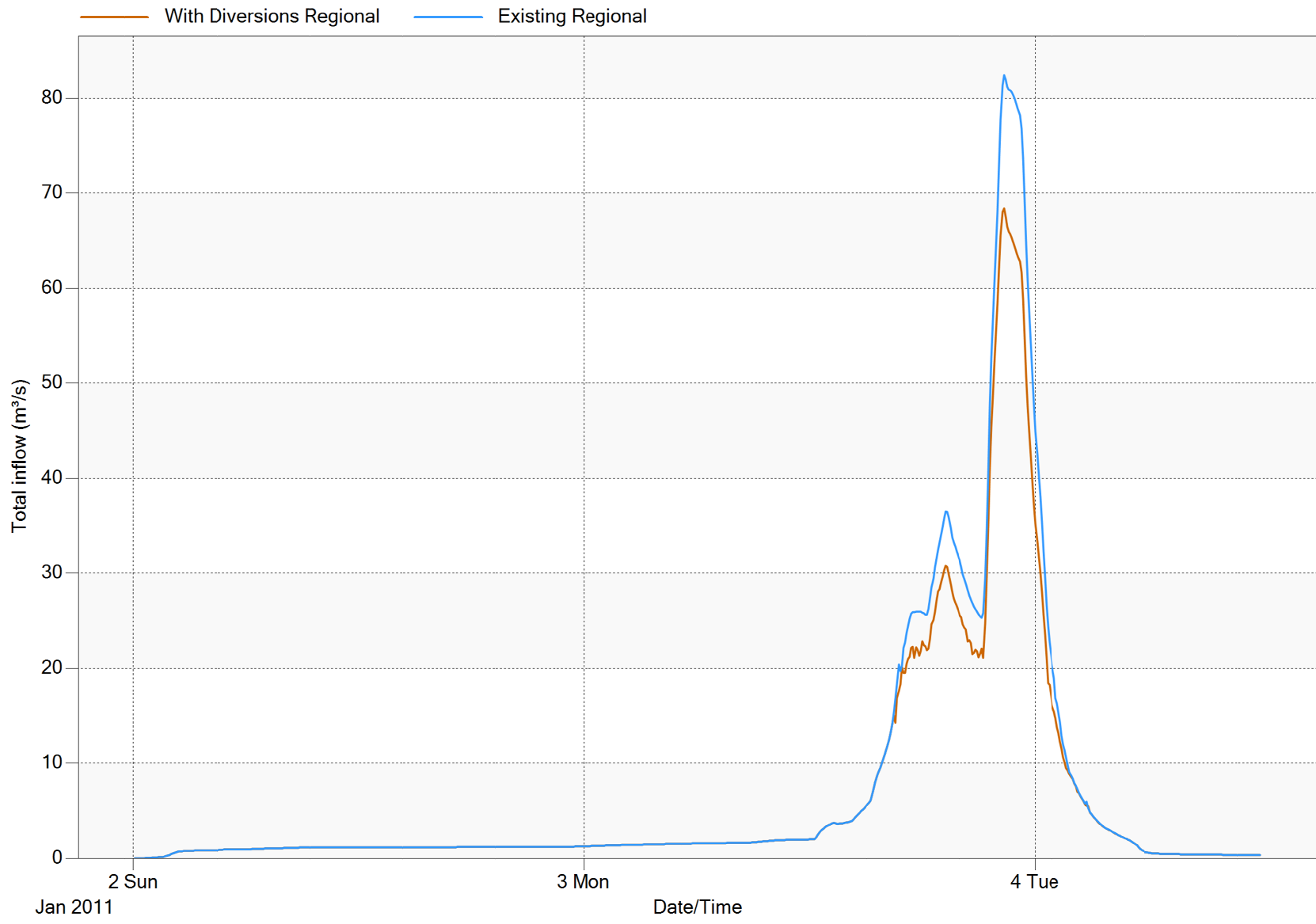
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APPENDIX

H-6 Conservation Halton Material regarding Bronte Creek

3.0 HYDROLOGY

3.1 General

As an initial step in delineating floodplain limits for the Bronte Creek, it was necessary to determine flood flows for selected frequencies along the major watercourses. The selected design storm frequencies were the 5, 10, 25, 50, 100 year and the Regional Storm events. Floodplain limits were based on flood flows for the Regional Storm under future land use conditions.

3.2 Hydrologic Simulation

Reference was made to previous hydrologic studies of Bronte Creek for basic hydrologic data and relationships. An existing hydrologic simulation model, "HYMO", was expanded and updated for the purpose of calculating peak flows in this study. Rainfall hyetograph data for the 5 to 100 year design storm events were determined from the Toronto International Airport and Hamilton Royal Botanical Gardens rainfall stations, and digitized according to the U.S. Soil Conservation Service Type II rainfall distribution over a 12 hour duration. The Regional Storm rainfall pattern for this part of Ontario is based upon Hurricane Hazel. To account for spatial averaging of the rainfall over the watershed, a reduction factor based upon the size of the watershed was applied. Both of these criteria were obtained from the Ministry of Natural Resources.

These rainfall amounts were transformed into runoff hydrographs at selected locations along the watercourses on the basis of local hydrologic characteristics. Hydrographs routed through the stream valleys were added to discharges from successive downstream subcatchments to obtain flow estimates at selected locations along the river.

Runoff from the watershed is directly related to drainage area, soils, land use, topography, vegetation, and storage in the form of lakes and swamps.

These factors were combined and interpreted using U.S. Soil Conservation Service techniques to determine runoff potential for each subcatchment. Appropriate changes to the runoff indices were made to account for the effect of the impervious urban areas on runoff volumes. Figure 3 shows the various subwatersheds included in the analysis. In addition, hydrologic parameters defining the runoff response times of the drainage areas, namely the time to peak (tp) and recession constant (K), were required.

These values are most commonly estimated from empirical relationships developed on similar drainage basins or calibrated against local streamflow data. The time to peak (tp) and the recession constant (K) were calculated within the HYMO model using William's 2 - parameter formulae:

$$K = 16.1A^{0.24}(H/L)^{-0.84}$$

$$tp = 6.54A^{0.39}(H/L)^{-0.5}$$

where A is the subwatershed area (sq.mi.) and H/L is the watershed height (ft) to length (mi) ratio.

The main hydrologic parameters used for simulation purposes are summarized on Tables 1A and 1B. These parameters were found to be suitable in a previous study of this watershed through calibration against a recorded flood hydrograph. There is only a relatively short streamflow record for Bronte Creek, and sufficient data for a complete calibration/validation was not available.

3.3 Mountsberg Dam

The lake created by Mountsberg Dam has a surface area of approximately 500 ac., and it has significant impacts on storing part of the flood and reducing downstream flows. The watershed area that contributes runoff to this lake is sub-area 2, which has a size of 14.25 sq.mi. Flood hydrographs generated from this area were routed in the HYMO model in order to simulate the storage effects.



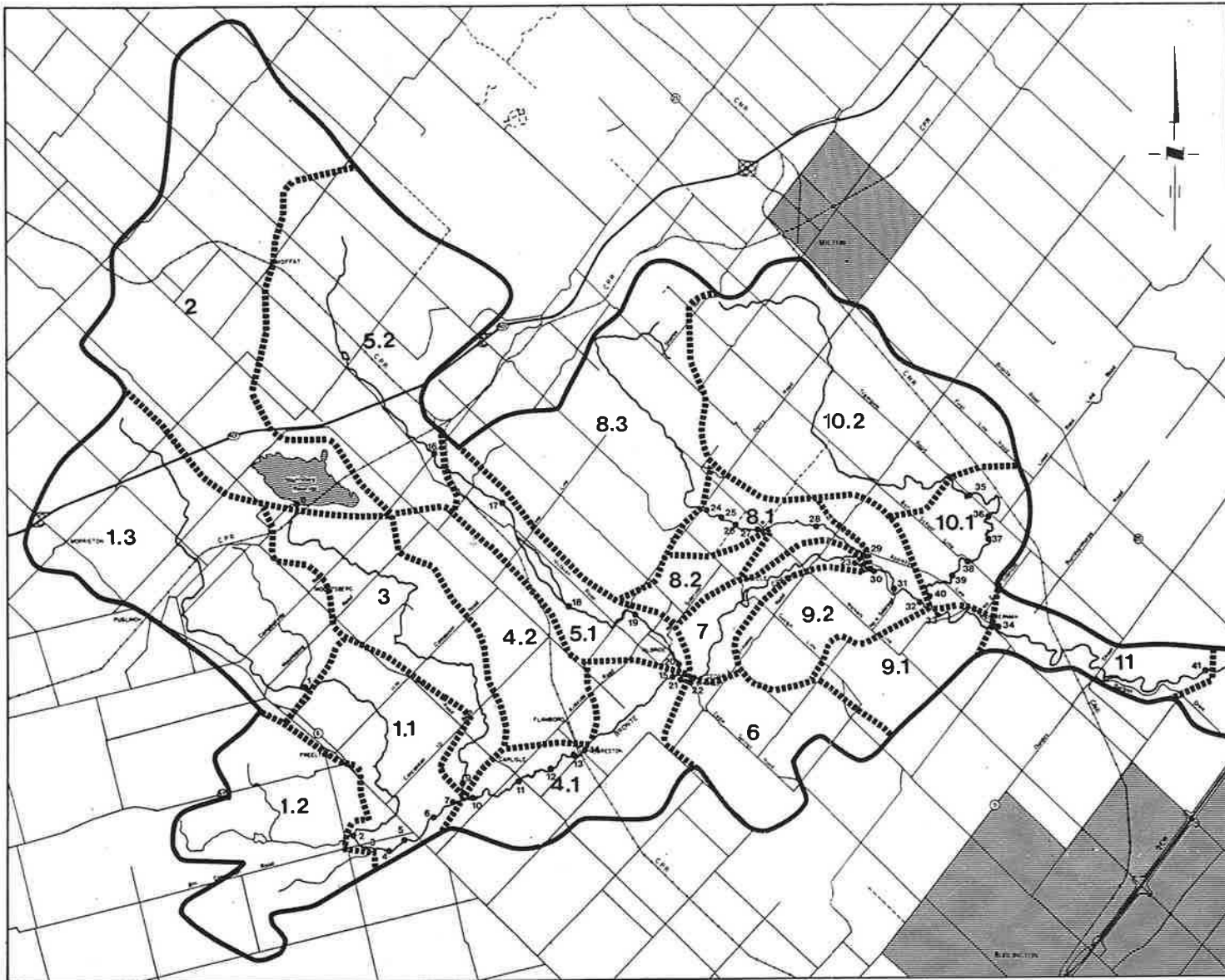
THE HALTON REGION
CONSERVATION AUTHORITY

BRONTE CREEK
FLOODPLAIN MAPPING STUDY

SUBCATCHMENT AREAS

LEGEND

- WATERSHED BOUNDARY
- SUBCATCHMENT BOUNDARY
- FLOW REFERENCE POINT



Proctor & Redfern Limited
Consulting Engineers and Planners
Toronto

FIGURE 3

TABLE 1B

SUMMARY OF SUBWATERSHED PARAMETERS

WATERSHED	AREA SQ MI	CN	WT SLOPE FT/MI	WT HT FT	LENGTH MI	TP HR	K HR	K/TP	B
1	24.70	69.0	16.47	179.	10.89	6.09	7.8	1.3	264.2
2	14.25	69.0	22.42	156.	6.94	4.00	5.2	1.3	262.0
3	5.50	61.0	19.95	124.	6.23	3.11	5.0	1.6	220.9
4	10.30	62.0	42.29	177.	4.18	2.37	2.7	1.1	291.5
5	15.50	62.0	45.15	474.	10.50	3.31	3.4	1.0	318.8
6	4.45	75.0	50.50	163.	3.24	1.60	2.0	1.3	268.8
7	1.90	71.0	31.44	114.	3.63	1.61	2.7	1.7	210.6
8	14.30	67.0	54.83	398.	7.26	2.68	2.6	1.0	329.3
9	6.70	75.0	69.00	283.	4.10	1.67	1.7	1.1	310.0
10	15.50	75.0	26.51	243.	9.15	4.08	4.9	1.2	277.9
11	1.80	74.0	25.22	123.	4.89	1.90	3.5	1.8	198.3
1.1	5.35	69.0	16.87	133.	7.89	3.55	6.0	1.7	211.5
1.2	7.15	69.0	21.41	47.	2.21	2.47	3.8	1.5	230.3
1.3	12.20	69.0	20.78	108.	5.21	3.66	5.0	1.4	250.2
4.1	2.50	62.0	38.85	35.	1.90	1.87	3.63	1.9	262.3
4.2	7.80	62.0	50.41	310.	4.50	1.78	1.82	1.0	272.3
5.1	3.30	62.0	42.25	247.	5.84	1.87	2.6	1.4	248.0
5.2	12.20	62.0	28.91	144.	4.97	3.11	3.8	1.2	271.9
8.1	2.00	67.0	38.71	150.	3.87	1.51	2.4	1.6	224.0
8.2	1.10	67.0	50.18	99.	1.97	.94	1.5	1.6	217.2
8.3	11.20	67.0	48.01	303.	6.31	2.56	2.7	1.1	306.6
9.1	2.97	75.0	69.18	278.	4.02	1.31	1.6	1.2	275.4
9.2	3.73	75.0	70.43	278.	3.95	1.38	1.6	1.2	285.9
4 (Upper)	2.5	62.0		35.	1.90	1.87	3.63		
4 (Lower)	7.8	62.0		310.	4.50	1.78	1.82		

TP and B Calculated by HYMO Formulas

3.4 Design Flows

Design flows for the 5 year through the Regional Storm event are presented in Table 2. In estimating these flows, the effect of storage at road crossings was neglected, as is the usual practice when dealing with floods for regulatory purposes.

TABLE 2
SUMMARY OF PEAK FLOOD FLOWS



DRAINAGE SUB-AREA	LOCATION	*	CROSS- SECTION NUMBER	FLOOD FLOWS IN CFS					
				REGIONAL	100 (Yr.)	50 (Yr.)	25 (Yr.)	10 (Yr.)	5 (Yr.)
1	- U/S Study Limit of Bronte Creek	1	1.91	4279	723	574	435	273	166
	- U/S of Sub-area 1.2	2	1.42	4279	723	574	435	273	166
	- D/S of 8th Concession Rd.	3	1.35	4465	770	610	465	290	175
	- Between 8th Concession & Hwy.6	4	1.27	4810	815	650	490	310	190
	- U/S of Hwy. 6	5	1.18	5160	865	685	520	325	200
	- D/S of Hwy. 6	6	1.05	5510	910	725	550	345	210
	- Outlet	7	1.00	5856	959	761	578	363	220
3	- U/S Study Limit of Mountsberg Trib.	8	3.61	3199	209	136	103	54.5	25.6
	- Outlet	9	3.00	3263	209	148	103	54.5	25.6
4	- U/S of Carlisle	10	4.49	8428	1114	876	658	405	240
	- Earth Embankment	11	-	-	1183	928	695	426	250
	- Section 13 (Previous Mapping)	12	-	8429	-	-	-	-	-
	- CPR Embankment	13	-	8903	-	-	-	-	-
	- U/S of Sub-area 4.2	14	4.12	8903	1199	931	695	426	250
	- Outlet	15	4.04	9591	1346	1030	765	441	250
5	- U/S Study Limit of Kilbride Trib.	16	5.78	4729	572	431	305	167	82
	- U/S of Base Line Road	17	5.58	5065	625	470	335	180	90
	- U/S of Derry Road	18	5.43	5400	675	510	360	195	97
	- D/S of Derry Road	19	5.22	5740	725	550	390	210	104
	- Outlet	20	5.01	6076	780	588	416	226	112
7	- Old Mill Pond								
	- U/S of Cedar Springs (incl. Area 5)	21	7.57	15329	2112	1609	1164	635	329
	- Just U/S of Cedar Springs								
	- Community (incl. Area 6)	22	7.48	16689	2573	1978	1425	813	453
	- Outlet	23	7.01	17152	2608	2001	1458	838	471
8	- U/S Study Limit of Lowville Trib.	24	-	5713	942	738	548	331	193
	- D/S of Derry Road	25	-	6065	1010	790	585	350	200
	- D/S of Derry Road	26	-	6420	1075	840	620	370	210
	- U/S of Sub-area 8.2	27	-	6772	1139	888	657	393	221
	- U/S of Britannia Road	28	-	7010	1180	925	685	410	235
	- Outlet	29	-	7245	1226	958	711	432	252
9	- WSC Gauge	30	9.33	23321	3743	2885	2112	1249	716
	- D/S of No. 4 Sideroad	31	9.27	24200	3940	3050	2240	1340	780
	- U/S of Indian Creek	32	9.17	25067	4140	3216	2370	1435	842
	- D/S of Indian Creek	33	9.13	30952	5729	4513	3402	2127	1303
	- Zimmerman	34		32581	6123	4838	3650	2299	1416
10	- U/S Study Limit of Indian Creek	35	10.44	5320	1369	1127	903	628	433
	- (limit of previous mapping)								
	- D/S of Tremaine Road	36	10.35	5715	1485	1220	980	660	470
	- U/S of Bell School Line	37	10.28	6100	1595	1320	1055	730	505
	- Bell School Line	38	10.19	6490	1710	1415	1130	785	540
	- U/S of Appleby Line	39	10.14	6880	1825	1510	1205	825	575
	- Outlet of Indian Creek	40	10.01	7266	1940	1605	1282	888	608
11	- Outlet - D/S Study Limit	41	11.00	31300	6123	4838	3650	2299	1416

* Flow Reference Pt. (See Figure 3)

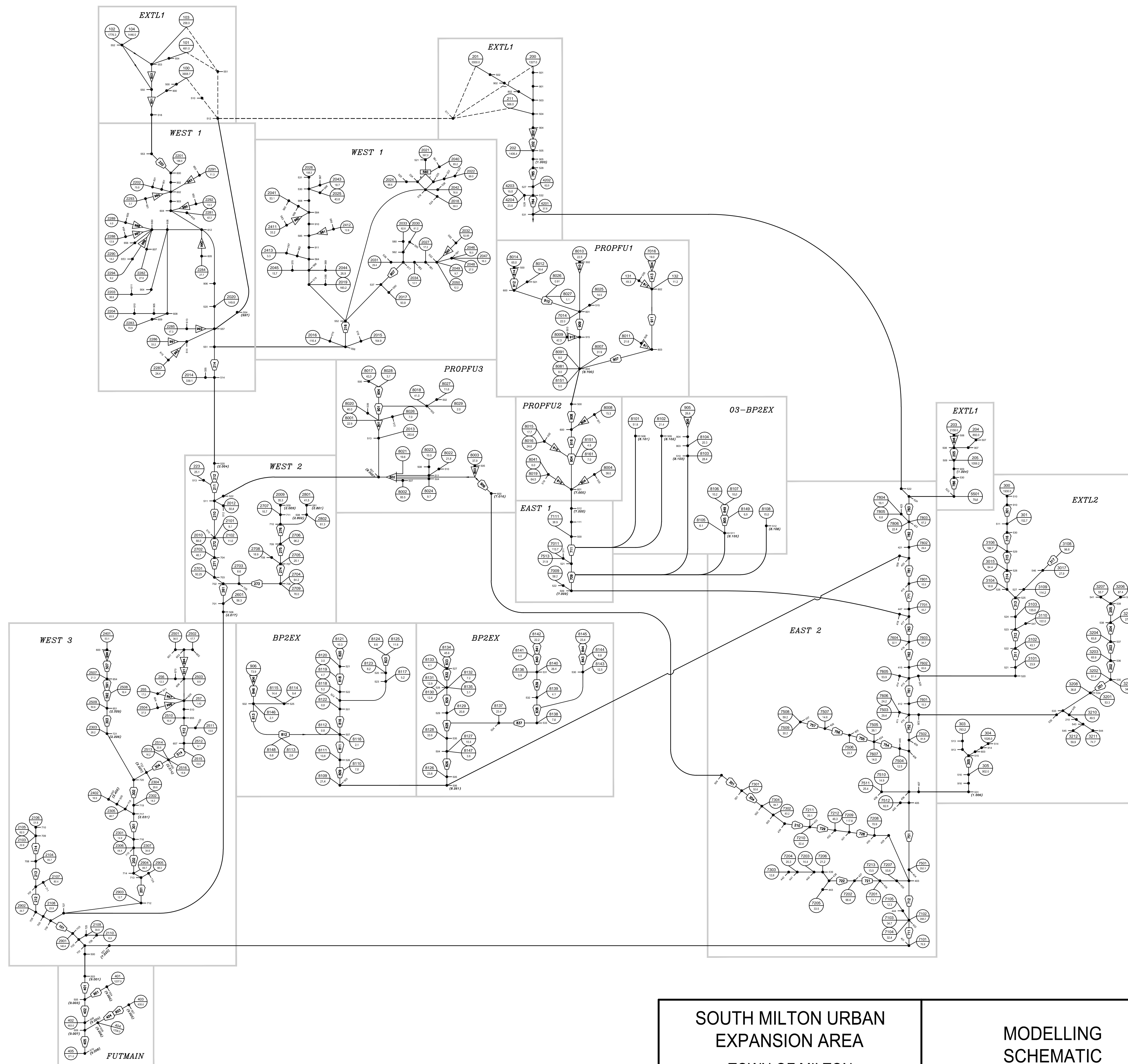
APPENDIX

H-7 South Milton Excerpts

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Plotted By: marykelly
 Last Saved By: marykelly

2017-03-17
 Last Saved: 2017-03-17



LEGEND

- SUB-AREA NUMBER
AREA (ha)
- RESERVOIR/
SWM FACILITY
- NODAL POINT
- STREAM REACH
(Reach Number)
- DATA SET NUMBER
(DSN)
- INPUT DATA FILENAME
- SPILLWAY
- GROUNDWATER

SCALE VALID ONLY FOR
24"x36" VERSION

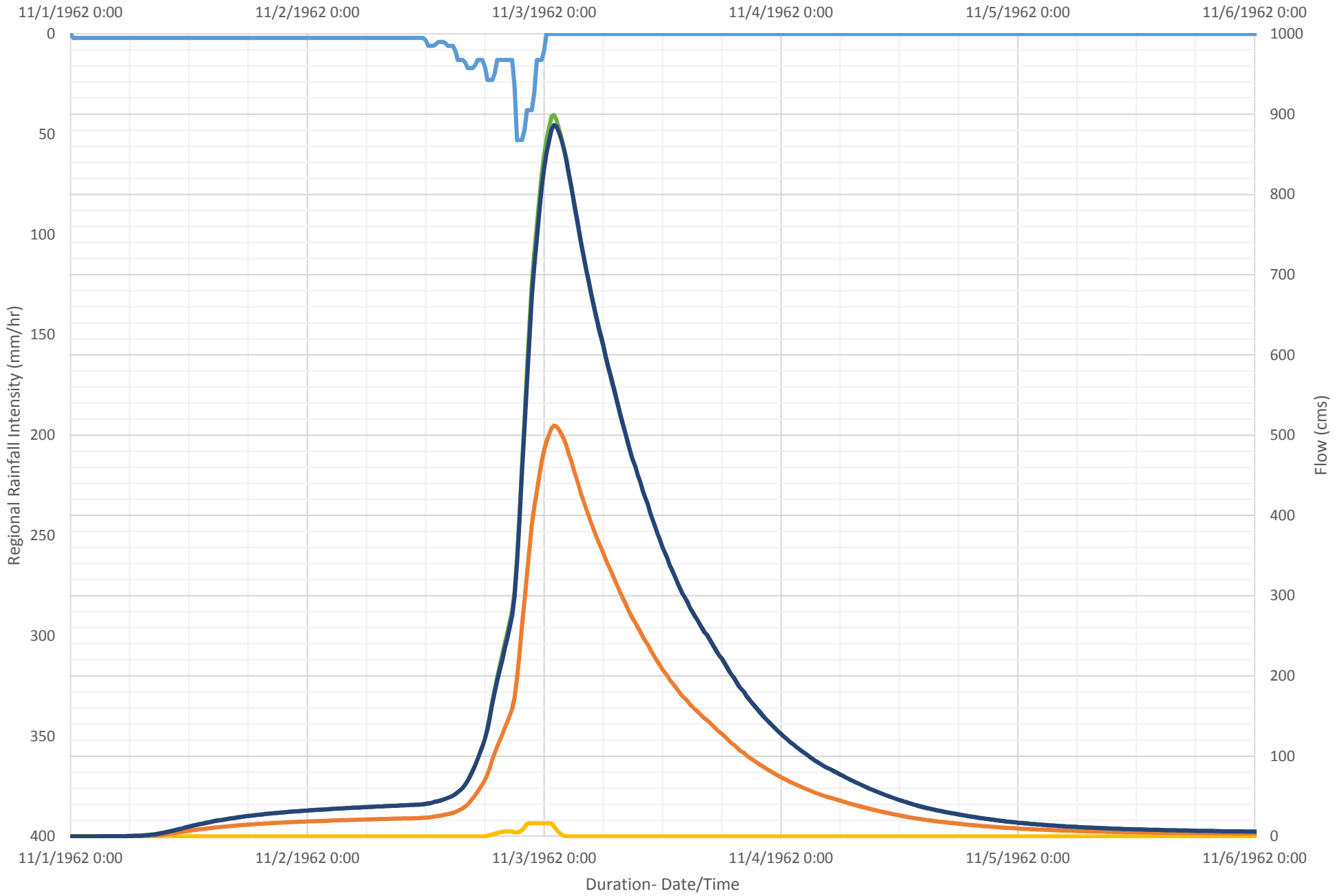
**SOUTH MILTON URBAN
 EXPANSION AREA
 TOWN OF MILTON**

**MODELLING
 SCHEMATIC**



Scale	N/A
Consultant File No.	TP116007
Figure No.	4.4.7

Surrogate Hydrograph with Diversion (Adjusted by Flow)



- Fourteen Mile Creek Diversion
- Sixteen Mile Creek East Branch
- Surrogate Sixteen Mile Creek East Branch + Diversion
- Surrogate Sixteen Mile Creek East Branch
- Regional Rainfall (Hurricane Hazel)

APPENDIX

H-8 Appendix C Drawings from 2023 Assessment